E. Coli TMDLs in Colorado: Finding Solutions

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2019 CASFM Crested Butte, CO















MILE HIGH FLOOD DISTRICT

engineers | scientists | innovators

Overview

- What's the problem with E. coli?
- Regulatory Basics
- Resources for MS4s
- Case Studies:
 - Fort Collins
 - Boulder





What's the problem with E. coli: 10 Issues

- 1. Recreating in waters with fecal contamination can make you sick.
- 2. Over 90 stream segments in Colorado are listed as impaired or on M&E for E. coli.
- 3. It's easy to exceed the E. coli stream standard, particularly in the summer.
- 4. Interpretation of monitoring data can be difficult or inconclusive.
- 5. We don't live in a sterile environment—E. coli can originate from both natural and humanrelated sources. Health risk from sources may vary.

Integrated Water Quality Monitoring and Assessment Report 2018





COLORADO Water Quality Control Division Department of Public Health & Environment

What's the problem with E. coli: 10 Issues

- 6. Solutions cross multiple disciplines and local government departments.
- 7. Finding the source(s) can be challenging and expensive.
- 8. Treating dry and wet weather runoff can be challenging and expensive.
- 9. In urban areas, there may not be a "silver bullet" that solves the problem.
- 10. Can the stream standard be consistently attained? If so, how? What are the MS4 permit implications if it's not?



Regulatory Relationships in Colorado: Standards-TMDLs-Permits



Colorado Stream Standards

- Fecal indicator bacteria vs. pathogens (e.g., *E. coli* O157:H7)
- EPA 2012 Recreational Water Quality Criteria
- Colorado stream standards
 - Magnitude: 126 cfu/100 mL (primary contact)
 - Duration: 61-day rolling geometric mean
 - Frequency: Geometric mean not allowed to exceed standard
- 303(d) List updated biennially: over 90 segments in Colorado impaired or on M&E list for *E. coli*

Colorado Use Classification	<i>E. coli</i> (cfu/100 mL)
Class E - Existing Primary Contact	126
Class P - Potential Primary Contact	205
Class N - Not Primary Contact	630
Class U - Undetermined	126

Total Maximum Daily Loads (TMDLs) & Implications for MS4s TMDL = ΣWLA + ΣLA + MOS

Where:

- WLA = the sum of wasteload allocations (point sources such as permitted wastewater and stormwater discharges)
- LA= the sum of load allocations (nonpoint sources and background)
- MOS=the margin of safety
- WWTPs typically not the source in Colorado
- MS4s likely to have requirements in CDPS permits due to TMDLs
- Nonpoint sources often significant



ervices & information Boards & commissions Divisions Concerns & emergencies

Total Maximum Daily Loads (TMDLs)

Back to clean water

TMDL prioritization

Development Information, five basic steps

Implementation Implementation of control information

Public notices Information, drafted notices

Arkansas River basin List of TMDL documents

Gunnison and Lower Dolores River basins List of TMDL documents

Rio Grande River basin

San Juan and Dolores River basins List of TMDL documents

South Platte River basin List of TMDL documents

Upper Colorado River basin List of TMDL documents

Table 3. Middle Reach <i>E. coli</i> TMDL: allowable loading and pollutant reductions necessary to meet the recreation based <i>E. coli</i> standard in Big Dry Creek.						
Loading Calculations (Giga-cfu/day)	High Flow	Moist Conditions	Mid-Range Flows	Dry Conditions	Low Flow	
TMDL	423.34	198.56	129.18	73.58	27.94	
MOS (10%)	42.33	19.86	12.92	7.36	2.79	
Allowable Load	381.01	178.71	116.26	66.22	25.14	
Exisiting Load	1119.13	425.48	244.05	114.49	94.98	
Required Reductions	66%	58%	52%	42%	74%	
WLA						
Westminster WWTF	58.24	54.32	51.49	31.97	16.99	
Broomfield WWTF	74.20	64.00	57.63	31.58	4.92	
MS4s	149.14	36.23	4.29	1.60	1.94	
Reserve Capacity	7.46	1.81	0.21	0.08	0.10	
LA						
Non-point Source	91.97	22.34	2.64	0.99	1.19	

Colorado E. coli Toolbox: A Practical Guide for Colorado MS4s

Introduction

- Colorado regulations
- Extent of problem
- TMDLs
- Finding the sources
- Developing a control strategy
 - Progression of controls
 - Modeling
- Source controls
- Structural BMPs
- Regulatory considerations/sitespecific standards

Colorado *E. coli* Toolbox: A Practical Guide for Colorado MS4s



Prepared by Wright Water Engineers, Inc. Geosyntec Consultants

Prepared for Urban Drainage and Flood Control District City and County of Denver

July 2016

www.udfcd.org

PATHOGENS in Urban Stormwater Systems



Prepared by Urban Water Resources Research Council Pathogens in Wet Weather Flows Technical Committee Environmental and Water Resources Institute, American Society of Civil Engineer

> With Support from Jrban Drainage and Flood Control District, Denver, CO Urban Watersheds Research Institute



E. coli Fact Sheets on UDFCD.org

- Regulatory Basics
- Microbial Source Tracking
- Stormwater BMP Performance
- Using GIS in Source Investigations

WHY DOES THE SOURCE OF E. COLI MATTER?

Elevated levels of fecal indicator bacteria (FIB), including E. coli, are one of the most common causes of water quality impairment in surface waters across the United States. While some sources of FIB can be identified through a sanitary survey, sources of FIB in many watersheds remain unknown and thus cannot be effectively controlled through targeted management actions. FIB results alone give no indication of the fecal source (human vs non- human). In addition, the health risk associated with exposure to water containing human waste is much greater than that of most non-human sources. Therefore, identifying the source(s) of fecal contamination in waters with chronically high FIB levels is of critical importance to meet recreational water quality criteria and reduce human health risk.

WHAT IS MICROBIAL SOURCE TRACKING?

Microbial source tracking (MST) uses a set of tools that allow for sources of fecal waste to be distinguished. These tools include conventional methods (e.g., ammonia, CCTV, dye testing) that have been used to identify illicit discharges for the past 20+ years, as well as more recently developed advanced laboratory methods that measure DNA specific to humans and other animals known as "markers" to identify sources. However, MST is more than just a set of tools or methods. MST is a process by which potential waste sources are systematically tested and investigated to identify and locate the origin of fecal bacteria in a contaminated water.





Protecting People, Property and the Environment



International Stormwater BMP Database

Home Get Data - Submit Data - Documents - Guidance - About -





Hamilton East Ecoroof, City of Portland Bureau of Environmental Services

Urban Stormwater Research Reports

- 2014 BMP Performance Summaries
- 2014 Statistical Appendices
- 2012 Manufactured Device Performance Analysis Summary
- 2012 Volume Reduction in Bioretention
- 2012 Database Overview
- 2012 Chesapeake Bay BMP Performance Summary

Welcome! The International Stormwater Best Management Practices (BMP) Database project website features a database of over 530 BMP studies, performance analysis results, tools for use in BMP performance studies, monitoring guidance and other study-related publications. New to the site? Start Here

News

- Coming Soon: Stream Restoration Database
- Agricultural BMP Database Initial Summary Report
- 2014 BMP Database Release
- 2014 BMP Performance Summaries
- 2013 Advanced Analysis
- National Stormwater Quality Database Has A New Home

Q Related Databases & Research

- Stream Restoration Database
- National Stormwater Quality Database
- Agricultural BMP Database
- Chesapeake Bay Research Portal
- A Retrieve Urban Stormwater BMP Performance
- BMP Study Retrieval Tool
- BMP Map Tool
- BMP Category Reports
- Online Statistical Analysis Tool
- Download Access Database



www.waterrf.org



U.S. Department of Transportation

Federal Highway Administration













Developing an E. coli Control Strategy

General Themes:

- Find the source(s)
- Address human source first, then other sources
- Address dry weather first, then wet weather
- Implement nonstructural/ source controls, then structural



City of Fort Collins Case Study



Fort Collins E. coli Impairments

Segment ID	Stream Segment Description	Season of
		Impairment
COSPCP11	Cache La Poudre from Shields St. to above Boxelder Creek	Not specified
COSPCP12	Cache La Poudre from above Boxelder Creek to South Platte River	May-October
COSPCP13a	Spring Creek and Fossil Creek (May-October)	May-October
COSPCP13b	Boxelder Creek from source to confluence with Cache La Poudre	Not specified



Starting Point

- Who is collecting data in your stream?
- Seasonal trends: Winter tends to attain standard; summer tends to exceed.
- Data availability: 5 samples over 61 days often not available
- Any major hot spots?

Table 7. All Sites Recreation Season (2014-2018)

2014-2018 Recreational Season Geometric Mean E. coli (MPN/100 mL)

	2014-2010 Recreational Season Geometric Mean E. Con (MPN/100 mL)					
Stream	Source	Site	Nbr.	Min	Max	Geomean
Boxelder Creek	CITYFTCO	BXC54	3	62	345	155
Boxelder Creek	CDPHE	BC-04	15	560	18000	2834
Boxelder Creek	CDPHE	BC-03	15	40	680	140
Boxelder Creek	CDPHE	BC01	3	249	870	455
Boxelder Creek	CDPHE	BC-02	15	20	697	126
Boxelder Creek	CITYFTCO	BXCSG	3	47	250	126
Boxelder Creek	CDPHE	BC-01	14	1	8800	116
Boxelder Creek	CDPHE	BC02	3	214	2420	506
Cache La Poudre	CITYFTCO	CLP287	27	10	9804	41
Cache La Poudre	CITYFTCO	432PLNC	39	13	1733	57
Cache La Poudre	CITYFTCO	390PPROS	39	26	3130	95
Cache La Poudre	CITYFTCO	380PNAT	39	28	2187	130
Cache La Poudre	CITYFTCO	370PBOX	39	25	3654	127
Cache La Poudre	CDPHE	CP03	3	88	206	127
Cache La Poudre	CDPHE	CP04	3	93	548	265
Cache La Poudre	CITYFTCO	325PFOS	39	16	1223	83
Fossil Creek	CITYFTCO	FOSC287	3	48	1553	307
Fossil Creek	CDPHE	FC-05	15	20	1520	184
Fossil Creek	CDPHE	FC-04	15	1	3460	189
Fossil Creek	CDPHE	FC-03	15	1	1040	210
Fossil Creek	CDPHE	FC-02	15	60	11100	516
Fossil Creek	CITYFTCO	FOSC34	3	276	1300	498
Fossil Creek	CDPHE	FC-00	15	1	190	13
Fossil Creek	CDPHE	FC-01	15	1	200	33
Spring Creek	CDPHE	SC-07	15	20	2580	153
Spring Creek	CDPHE	SC-06	15	40	640	140
Spring Creek	CDPHE	SC-05	15	40	800	120
Spring Creek	CDPHE	SC-04	15	40	780	142
Spring Creek	CITYFTCO	SPRC287	3	365	1120	631
Spring Creek	CDPHE	SC-03	15	1	1140	176
Spring Creek	CDPHE	SC-02	15	20	1060	145
Spring Creek	CITYFTCO	SPRCEP	3	34	1986	214
Spring Creek	CDPHE	SC-01	15	1	700	133

Dry Weather Investigation

- Upload mapped outfalls into Collector App
- How many days to find 150 outfalls?
- Estimating flow rate
- Uploading photos
- Identifying suspect outfalls
- Getting samples to lab on time
- Car or bike or both



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	Take Photo		Ø Attach			
Visite	d		>			
Flow	Rate (gpm)					
Samp	led		>			
Suspe	ect		>			
Notes Propo	Notes Proposed poudre sample location					

Field Challenges













Collecting GIS Data

- Infrastructure age and maintenance history
- BMP locations
- Ditches
- Septic systems
- Hobby farms, large animal facilities/properties
- Natural areas
- Homeless/transient densities



http://www.homeward2020.org

POPULATION DASHBOARD

Housing First Initiative: Addressing Long-Term Homelessness in Fort Collins

Housing First Initiative (HFI) seeks to produce actionable and accessible community-level data on the issue of homelessness. With that data, we can better identify and implement solutions for people experiencing homelessness. HFI collects and reports data on persons experiencing homelessness in Fort Collins for six months or longer, pilots housing first solutions through local partnerships, and provides intensive case management to transition participants from homelessness to housing. "This dashboard does not provide data on all persons experiencing homelessness, but focuses on individuals experiencing chronic and long-term homelessness.

Current # of persons experiencing homelessness 6 months or longer in Fort Collins: 434



City of Boulder Case Study

STORMWATER PROGRAM TIMELINE



Current Stormwater Quality Program

Illicit Discharge Program	Monitoring	TMDL Development	TMDL Implementation Plan Development	TMDL Implementation n Plan Update
				(
Slow the Flow Efforts	"Doo Good" Outreach	Pet Waste Stations	Boulder Creek Outfall Surveys	Storm & Sanitary Efforts
Stormwater Master Plan Update	Investigation of Sanitary Mains Under the Creek	Memo: Raccoons in storm drains	Raccoon- proofing Pilot	GI Strategic Plan
Special Monitoring Studies	Neonic Sampling	Macroinvert. Monitoring	Instream Flow Program	Bear-proof Trash Enclosure









Boulder Creek, Colorado Segment 2b: From 13th Street to the Confluence with South Boulder Creek

Total Maximum Daily Load Escherichia coli



Infrastructure Investments

1.3 Million Dollars to inspect, clean and line the storm sewer system

Million Dollars to line the sanitary system in the University Hill and Downtown areas

Sewer Mains TVed under Boulder Creek

Cross-Connections Identified to Date

2



Bear Trash Enclosure Program



What Can You Do?







Monitoring

TMDL Implementation Plan













Conclusions

- E. coli: coming soon to an MS4 permit near you!
- Where to start: Better understand the source(s) during dry weather.
- Expect messy data.
- BMPs can help reduce bacteria—this does not always mean attainment of instream standards at end of pipe though.
- Colorado-based resources are available to develop a better understanding of various aspects of bacteria issues.
- Partners are important—solutions likely extend beyond public works departments.

Questions?

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Development of a New Integrated Decision Support Tool (i-DST)

ELIZABETH GALLO

SWURDHINER I.A. CONSOL INISOL

US-EPA NATIONAL PRIORITIES

https://idst.mines.edu/



Outline



- General Background
- i-DST Background
- Watershed Scale Module
- Front-end Graphical User Interface (GUI)
- Stormwater Control Measure (SCM) and Optimization Simulator
- Test Site: Berkeley Neighborhood Watershed

About me

- 3rd year PhD Candidate at Colorado School of Mines
- Completed my masters in 2017
- Hydrologic Science and Engineering Program
- Civil and Environmental Engineering Department
- Advisor: Terri S. Hogue




Motivation

Urbanization stresses drainage networks and aquatic ecosystems





Image from Piotr Redlinski for The New York Times



Motivation

Urbanization stresses drainage networks and aquatic ecosystems______

Pollution Rain Runoff **Pollution** Traditional solutions are "grey" infrastructure, which is aging/undersized.

Image from Piotr Redlinski for The New York Times



Solution





Figure: DC Water (<u>https://www.dcwater.com/green-infrastructure</u>)

Research Focus





Previous work:

Variability in achieving water quality compliance with SCMs impacts the decision making process when optimizing for a watershed.



| Baseline*
1 | No New | Capture (m ³) | (1 2) | and the second se | COSt
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 | (TMDL
 | Con | pliance [| %])** | *
 | Davs/vr | | (TMDL | Con
 | pliance [| %])** | * |
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| Baseline*
1 | No New | Capture (m ⁻) | (km) | Reduction | (Billions)
 | **
 | 0 | Cu DW
 | F | b DW | 1 | Zn DW
 | ** | Cu WW | | | |
 | Pb WW | | Zn WW | |
| 1 | | 0 | | |
 | 259
 | 86 | (66.8)
 | 0 | (100.0) | 0 | (100.0)
 | 106 | 105 | (0.9)
 | 0 | (100.0) | 19 | (82.1 |
| | All | 1,358,766 | 3.52 | 43.0% | 0.55
 | 336
 | 0 | (100.0)
 | 0 | (100.0) | 0 | (100.0)
 | 29 | 10 | (65.5)
 | 0 | (100.0) | 1 | (96.6 |
| 2 | IT | 3,094,830 | 2.02 | 95.0% | 0.70
 | 354
 | 0 | (100.0)
 | 0 | (100.0) | 0 | (100.0)
 | 11 | 10 | (9.1)
 | 0 | (100.0) | 8 | (27.3 |
| 3 | DP | 3,094,830 | 2.02 | 10.0% | 0.69
 | 339
 | 0 | (100.0)
 | 0 | (100.0) | 0 | (100.0)
 | 26 | 6 | (76.9)
 | 0 | (100.0) | 0 | (100.0 |
| 4 | VS+BR+PP | 2,798,910 | 11.18 | 65.0% | 1.40
 | 340
 | 0 | (100.0)
 | 0 | (100.0) | 0 | (100.0)
 | 25 | 11 | (56.0)
 | 0 | (100.0) | 2 | (92.0 |
| 5 | VS+BR+PP | 314,415 | 1.24 | 11.0% | 0.20
 | 278
 | 73 | (73.7)
 | 0 | (100.0) | 0 | (100.0)
 | 87 | 84 | 3.4
 | 0 | (100.0) | 18 | (79.3 |
| Baseline* | No New | 0 | - | | 1 E 13
 | 333
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 | 32 | 18 | (43.8)
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| 1 | BR | 2,901,249 | 6.42 | 69.6% | 1.51
 | 357
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 | 8 | 7 | (12.5)
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| 1b | BR+PP | 2,901,249 | 6.68 | 55.7% | 1.51
 | 359
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 | - | | 3 |
 | 6 | 5 | (16.7)
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| 2 | VS+DP | 2,901,249 | 5.54 | 4.4% | 0.71
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 | 11 | 11 | (0.0)
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| 2b | VS+DP+PP | 2,901,249 | 7.04 | 34.0% | 0.89
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| 3 | VS+IT | 2,901,249 | 5.41 | 45.4% | 0.70
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 | - | - 14
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| Зb | VS+IT+PP | 2,901,249 | 6.92 | 55.3% | 0.90
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| Baseline* | No New | 0 | ~ | 31 |
 | 333
 | 307 | (89.8)
 | 127 | (95.8) | 214 | (35.7)
 | 32 | 6 | (81.3)
 | 2 | (93.8) | 14 | (56.3 |
| 1 | BR | 12,818,268 | 27.97 | 47.0% | 6.60
 | 358
 | 68 | (97.9)
 | 51 | (98.4) | 18 | (95.0)
 | 7 | 1 | (85.7)
 | Ó | (100.0) | 6 | (14.3 |
| 1b | BR+PP | 12,818,268 | 15.02 | 53.0% | 6.80
 | 360
 | 71 | (97.8)
 | 53 | (98.4) | 18 | (95.0)
 | 5 | 2 | (60.0)
 | 0 | (100.0) | 5 | (0.0) |
| 2 | VS+DP | 12,818,268 | 37.30 | 29.0% | 3.80
 | 350
 | 62 | (98.0)
 | 47 | (98.5) | 15 | (95.7)
 | 15 | Ó | (100.0)
 | 0 | (100.0) | 3 | (80.0 |
| 2b | VS+DP+PP | 12,818,268 | 24.86 | 46.0% | 5.20
 | 358
 | 69 | (97.9)
 | 52 | (98.4) | 18 | (95.0)
 | 7 | 1 | (85.7)
 | ٥ | (100.0) | 6 | (14.3 |
| 3 | VS+IT | 12,818,268 | 37.30 | 55.0% | 3,80
 | 361
 | 75 | (97.7)
 | 57 | (98.2) | 19 | (94.7)
 | 4 | 0 | (100.0)
 | 0 | (100.0) | 2 | (50.0 |
| 3b | VS+IT+PP | 12,818,268 | 24.86 | 57.0% | 5.20
 | 361
 | 75 | (97.7)
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W Days/yea | 3DP4VS+BR+PP5VS+BR+PPiaseline*No New1BR1bBR+PP2VS+DP2bVS+DP+PP3VS+IT3bVS+IT+PPJaseline*No New1BR1bBR+PP2VS+DP2bVS+DP2bVS+DP2bVS+DP3VS+IT3bVS+IT3bVS+IT3bVS+IT+PP* Baseline ScenaryW Days/year equals the | 3 DP 3,094,830 4 VS+BR+PP 2,798,910 5 VS+BR+PP 314,415 iaseline* No New 0 1 BR 2,901,249 1b BR+PP 2,901,249 2 VS+DP 2,901,249 2b VS+DP 2,901,249 3b VS+IT 2,901,249 3a VS+IT 2,901,249 3b VS+IT 2,818,268 1b BR 12,818,268 2 VS+DP 12,818,268 3 VS+IT 12,818,268 3b VS+IT 12,818,268 < | 3 DP 3,094,830 2.02 4 VS+BR+PP 2,798,910 11.18 5 VS+BR+PP 314,415 1.24 iaseline* No New 0 - 1 BR 2,901,249 6.42 1b BR+PP 2,901,249 6.68 2 VS+DP 2,901,249 5.54 2b VS+DP 2,901,249 5.41 3b VS+IT 2,901,249 6.92 Jaseline* No New 0 - 1 BR 12,818,268 27.97 1b BR+PP 12,818,268 37.30 2b VS+DP 12,818,268 37.30 2b VS+DP 12,818,268 37.30 2b VS+DP+PP 12,818,268 37.30 2b VS+IT 12,818,268 37.30 3b VS+IT 12,818,268 24.86 3 VS+IT 12,818,268 24.86 * Baseline Scenario is | 3 DP 3,094,830 2.02 10.0% 4 VS+BR+PP 2,798,910 11.18 65.0% 5 VS+BR+PP 314,415 1.24 11.0% iaseline* No New 0 - - 1 BR 2,901,249 6.42 69.6% 1b BR+PP 2,901,249 6.68 55.7% 2 VS+DP 2,901,249 5.54 4.4% 2b VS+DP 2,901,249 5.41 45.4% 3b VS+IT 2,901,249 5.41 45.4% 3b VS+IT 2,901,249 6.92 55.3% iaseline* No New 0 - - 1 BR 12,818,268 27.97 47.0% 1b BR+PP 12,818,268 37.30 29.0% 2b VS+DP 12,818,268 37.30 29.0% 2b VS+DP+PP 12,818,268 37.30 55.0% 3b VS+IT </td <td>3 DP 3,094,830 2.02 10.0% 0.69 4 VS+BR+PP 2,798,910 11.18 65.0% 1.40 5 VS+BR+PP 314,415 1.24 11.0% 0.20 iaseline* No New 0 - - - 1 BR 2,901,249 6.42 69.6% 1.51 1b BR+PP 2,901,249 5.54 4.4% 0.71 2 VS+DP 2,901,249 5.54 4.4% 0.71 2b VS+DP 2,901,249 7.04 34.0% 0.89 3 VS+IT 2,901,249 5.41 45.4% 0.70 3b VS+IT 2,901,249 6.92 55.3% 0.90 iaseline* No New 0 - - - 1 BR 12,818,268 27.97 47.0% 6.60 1b BR+PP 12,818,268 37.30 29.0% 3.80 2b VS+D</td> <td>3 DP 3,094,830 2.02 10.0% 0.69 339 4 VS+BR+PP 2,798,910 11.18 65.0% 1.40 340 5 VS+BR+PP 314,415 1.24 11.0% 0.20 278 iaseline* No New 0 - - 333 1 BR 2,901,249 6.42 69.6% 1.51 357 1b BR+PP 2,901,249 6.68 55.7% 1.51 359 2 VS+DP 2,901,249 5.54 4.4% 0.71 354 2b VS+DP 2,901,249 7.04 34.0% 0.89 356 3 VS+IT 2,901,249 6.92 55.3% 0.90 358 iaseline* No New 0 - - 333 1 BR 12,818,268 27.97 47.0% 6.60 358 ibb BR+PP 12,818,268 37.30 29.0% 3.80 360<</td> <td>3 DP 3,094,830 2.02 10.0% 0.69 3.39 0 4 VS+BR+PP 2,798,910 11.18 65.0% 1.40 340 0 5 VS+BR+PP 314,415 1.24 11.0% 0.20 278 73 iaseline* No New 0 - - - 333 - 1 BR 2,901,249 6.42 69,6% 1.51 357 - 1b BR+PP 2,901,249 6.68 55.7% 1.51 359 - 2 VS+DP 2,901,249 5.54 4.4% 0.71 354 - 2b VS+DP+PP 2,901,249 5.41 45.4% 0.70 357 - 3a VS+IT 2,901,249 5.41 45.4% 0.70 357 - 3b VS+HT 2,901,249 6.92 55.3% 0.90 358 - 3aseline* No New 0 -<td>3 DP 3,094,830 2.02 10.0% 0.69 3.39 0 (100.0) 4 VS+BR+PP 2,798,910 11.18 65.0% 1.40 340 0 (100.0) 5 VS+BR+PP 314,415 1.24 11.0% 0.20 278 73 (73.7) iaseline* No New 0 - - 333 - - 1 BR 2,901,249 6.42 69.6% 1.51 357 - - 2 VS+DP 2,901,249 6.68 55.7% 1.51 359 - - 2 VS+DP 2,901,249 5.54 4.4% 0.71 354 - - 2b VS+DP+P 2,901,249 5.41 45.4% 0.70 357 - - 3b VS+IT 2,901,249 6.92 55.3% 0.90 358 - - 3b VS+IT 2,818,268 27.97 47.0%</td><td>3 DP 3,094,830 2.02 10.0% 0.69 339 0 (100.0) 0 4 VS+BR+PP 2,798,910 11.18 65.0% 1.40 340 0 (100.0) 0 5 VS+BR+PP 314,415 1.24 11.0% 0.20 278 73 (73.7) 0 iaseline* No New 0 - - 333 - - - 1 BR 2,901,249 6.42 69,6% 1.51 357 - - - 1b BR+PP 2,901,249 6.68 55.7% 1.51 359 - - - 2 VS+DP 2,901,249 5.54 4.4% 0.71 354 - - - 3 VS+IT 2,901,249 5.41 45.4% 0.70 357 - - - 3b VS+IT 2,901,249 5.41 45.4% 0.70 358 -</td><td>3 DP 3,094,830 2.02 10.0% 0.69 339 0 (100.0) 0 (100.0) 4 VS+BR+PP 2,798,910 11.18 65.0% 1.40 340 0 (100.0) 0 (100.0) 5 VS+BR+PP 314,415 1.24 11.0% 0.20 278 73 (73.7) 0 (100.0) iaseline* No New 0 - - 333 - - - - 1 BR 2,901,249 6.42 69,6% 1.51 357 - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - - -</td><td>3 DP 3,094,830 2,02 10.0% 0.69 339 0 (100.0) 0 (100.0) 0
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Stormwater management options and decision-making in the urbanized watersheds of Los Angeles, CA. *Journal of Sustainable Water in the Built Environment*. Gallo et al. In Press.

Previous work:

Variability in achieving water quality compliance with SCMs impacts the decision making process when optimizing for a watershed.



	BCDCLARLeast DegradedMost DegradedVariableNoYesNoVariableAnyTreat and Release OnlyTreat and release in more urbanized areas		
Water Quality	Least Degraded	Most Degraded	Variable
Eliminate TMDLs with SCMs?	BCDCLARalityLeast DegradedMost DegradedVariableTMDLs withYesNoVariableSCM OptionsAnyTreat and Release OnlyTreat and rele urbanized are	Variable	
Available SCM Options	Any	Treat and Release Only	Treat and release in more urbanized areas
Available Ancillary Benefits to Consider	All	Restricted to above options	Restricted in urban areas

Stormwater management options and decision-making in the urbanized watersheds of Los Angeles, CA. *Journal of Sustainable Water in the Built Environment*. Gallo et al. In Press.

Current work:

Stormwater practitioners lack a comprehensive decision support tool that incorporates both green and grey infrastructure, life cycle costs and environmental assessment, and co-benefits

Integrated Decision Support Tool (i-DST)



Conceptual model of tool components and the flow of information between them





i-DST Project Goals

- Develop an integrated, scalable, decision support tool (called i-DST) for grey, green, and hybrid infrastructure
- **Planning-level** tool suitable for project prioritization (not design)
- Components:
 - Hydrology, water quality
 - BMP optimization and uncertainty estimation
 - Life cycle cost assessment (LCCA)
 - Life cycle assessment (LCA)
 - Quantification of co-benefits







i-DST Project Team





fost

Science Advisory Board (SAB)

- Have a board of 21 urban water experts to help us:
 - Identify tool needs
 - Identify novel grey/green infrastructure techniques to include
 - Connect us with datasets
 - Provide feedback on the tool





- NGOs
- Consultants
- City/county governments
- Federal agencies
- Academics

Science Advisory Board (SAB)



Member	Organization	State
Janet Clements	Corona Environmental Consulting	СО
Darren Mollendor	City of Denver	СО
Holly Piza	Denver Urban Drainage and Flood Control District	СО
Tracey Pond	City of Golden	СО
Scott Struck	Geosyntec Consultants	СО
Jeffrey Williams	City of Denver	СО
Colin Bell	City and County of Denver: Green Infrastructure Program	СО

Scalable

- Tool will be developed at:
 - Watershed level (i-DST)
 - Individual site scale (i-DST-SB)





Site Scale

IDENTAL OF CHILDREN

14

Scalable

- Tool will be developed at:
 - Watershed level (i-DST)
 - Individual site scale (i-DST-SB)















EPA SUSTAIN Features





Also water quality treatment

$$Cost = a + b \cdot (Length)^{c} + d \cdot (Area)^{e} + f \cdot (Volume)^{g}$$

Built In* Evaluation Factors:

- Peak Discharge
- Annual Average Volume
- Exceeding Frequency
- Annual Average Load
- Annual Average Concentration
- Maximum Days Average Concentration







Image: Lee J., Riverson J.

<u>Scenario Builder</u>

Arc GIS 9.3

Excel VBA (Opti-Tool)

- EPA Region 1 Tool
- TetraTech
- GUI to build input files

QT/C++





Proje	-DST Wate
Proje	oct
rc GIS 9.3	Model Contr
	Output Dire
	Model Out
xcel VBA (Opti-1001)	GroupBox
	Start Date
	Timeserie:
	BMP Simul
	CRRAT:
	Watershed
	Watershed
1/0++	Direct Run
 Easier and faster 	Interstorm
 Less error prone More control 	

•	Po	rta	bil	lity

A

Ε

i-DST Watershed Tool		- 🗆 X
Project		
<< Back	General Sketch Routing Optimization Next >>	Create Input File
Model Controls Pollutants	Land Use Definitions Climate Timeseries	
Output Directories		
Land Output Directory:		Browse
Model Output Directory:		Browse
GroupBox		
Start Date of Simulation:	1/1/2000	
End Date of Simulation:	1/1/2000 Model Control Option	ns
Timeseries Timestep:	5 Same timestep a	as land timeseries
BMP Simulation Timestep:	5 O Hourly	
CRRAT:	1.5	
Watershed Timeseries		
Watershed Name	Watershed Percent Impervious: 0	
Direct Runoff Timeseries Fil	e: Brows	e
Interstorm Timeseries File:	Browse	2



1. Hybrid-Grey Infrastructure

Available SCM Types in SUSTAIN Green Roof Bio-retention Infiltration Trench Vegetated Swale Dry Pond Wet Pond Buffer Strip Porous Pavement Rain Barrel Cistern



1. Hybrid-Grey Infrastructure

* Simulated as a pipe or a 'box' SCM figures: StormTrap and Contech Available SCM Types in SUSTAIN Green Roof **Bio-retention** Infiltration Trench Vegetated Swale Dry Pond Wet Pond Buffer Strip Porous Pavement Rain Barrel **Underground Detention* Underground Infiltration* Underground Gravel Bed Above Ground Storage**



Underground Pipes

Underground Detention



Underground Infiltration



 Hybrid-Grey Infrastructure
 Simulation of Underground SCMs

1. Bypass when reach max SCM volume capacity



 Hybrid-Grey Infrastructure
 Simulation of Underground SCMs

1. Bypass when reach max SCM volume capacity



- Underground SCM
- Above ground SCM
- Pre SCM Implementation

 Hybrid-Grey Infrastructure
 Simulation of Underground SCMs

2. Accurate stage-surface area-volume relationship for pipes



Underground pipe

Above ground "box"

 Hybrid-Grey Infrastructure
 Simulation of Underground SCMs

2. Accurate stage-surface area-volume relationship for pipes



- 1. Hybrid-Grey Infrastructure
- 2. Simulation of Underground SCMs
- 3. BMP decay rate calibration tool



- 1. Hybrid-Grey Infrastructure
- 2. Simulation of Underground SCMs
- 3. BMP decay rate calibration tool
- 4. More evaluation factors

Water Quantity Benefits Average Annual Flow Volume Flow Exceedance Frequency Flow Duration Curve Peak Discharge Flow

Water Quality Benefits Average Annual Load Average Annual Concentration Days above X Concentration

- 1. Hybrid-Grey Infrastructure
- 2. Simulation of Underground SCMs
- 3. BMP decay rate calibration tool
- 4. More evaluation factors

Water Quantity Benefits Average Annual Flow Volume Flow Exceedance Frequency Flow Duration Curve Peak Discharge Flow Seasonal Average Flow Volume Average Annual Groundwater Recharge Seasonal Average Groundwater Recharge Average Annual Evapotranspiration Seasonal Average Evapotranspiration New Peak Discharge Flow Calculations

Water Quality Benefits

Average Annual Load Average Annual Concentration Days above X Concentration Seasonal Average Load Seasonal Average Concentration

- 1. Hybrid-Grey Infrastructure
- 2. Simulation of Underground SCMs
- 3. BMP decay rate calibration tool
- 4. More evaluation factors
- 5. Life Cycle costs utilized in the Optimizer

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Test Site: Berkeley Neighborhood Watershed

- 1,035 acres
- 53% impervious
- 15% of total area covered by future infil-redevelopment
- Concern of increase in surface water runoff



High Resolution Modeling of Infil Development Impact on Stormwater Dynamics in Denver, CO. *Journal of Sustainable Water in the Built Environment*. Panos et al. 2019

Test Site: Berkeley Neighborhood Watershed

What is the optimal suite and number of SCMs to reduce infil-redevelopment runoff back to pre-redevelopment flow?



Fig. 6. Percent increase in total surface runoff volume from baseline for each design storm event by scenario. The WQCV is a 6-h event, while all others are 24-h.

- Scenario 1: +1.2% imp
- Scenario 2: +4.7% imp
- Scenario 3: +8.1% imp

High Resolution Modeling of Infil Development Impact on Stormwater Dynamics in Denver, CO. *Journal of Sustainable Water in the Built Environment*. Panos et al. 2019
Test Site: Berkeley Neighborhood Watershed

> What is the optimal suite and number of SCMs to reduce infil-redevelopment runoff back to pre-redevelopment flow?



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What is a pareto curve?



- Plotting cost vs evaluation factor during the optimization
- Evolutionary search technique
- Solve nonlinear, single or multi-objective, complex problems
- Find the optimal solution



Pareto Curve Example





SCMs in the Berkeley Neighborhood model

	Los Angeles Capital Cost/ft ³	Area on Surface [ft ²]	Surface Storage Volume [ft ³]	Total Storage Volume [ft ³]	Maximum units to be optimized
Bio-retention	14.60	100	100	306	1300
Vegetated Swale	10.07	32	50	82	2600
Underground Infiltration	10.34	0	100	253	1300
Porous Pavement	15.69	100	100	204	1300

Optimization Set up



Individual SCM Comparison

- 4 pareto curves
- Optimize each SCM separately
- Routing 100% of infil-redevelopment to the SCM
- Evaluation factors:
 - average annual flow volume
 - groundwater recharge potential

Full optimization

- 1 pareto curve
- Optimize all SCMs simultaneously
- Routing 25% of infil-redevelopment to each SCM type
- Evaluation factors:
 - average annual flow volume
 - groundwater recharge potential

Results: Individual Optimizations

Individual SCM Comparison

- 4 optimization curves
- Optimize each SCM separately
- Routing 100% of infilredevelopment to the SCM



Underground infiltration reaches baseline conditions at the cheapest capital cost.



Tradeoff between cost and volume capacity



I argue that when comparing individual SCM pareto frontiers, displaying capture volume on the x-axis is efficient for planning level decisions.



I argue that when comparing individual SCM pareto frontiers, displaying capture volume on the x-axis is efficient for planning level decisions.



What about other benefits??



15+ pareto curve plots is a lot to look at and almost impossible to compare.

Water Quantity Benefits Average Annual Flow Volume Flow Exceedance Frequency Flow Duration Curve Peak Discharge Flow Seasonal Average Flow Volume Average Annual Groundwater Recharge Seasonal Average Groundwater Recharge Average Annual Evapotranspiration Seasonal Average Evapotranspiration New Peak Discharge Flow Calculations

> Water Quality Benefits Average Annual Load Average Annual Concentration Days above X Concentration Seasonal Average Load Seasonal Average Concentration

Can we summarize multiple evaluation factors in only one plot??

Example from Los Angeles modeling efforts



$$Benefit \ Score_{j,i} = \left[\frac{Benefit \ value_{j,i} \ -\min[Benefit \ Value_i]}{\max[Benefit \ Value_i] - \min[Benefit \ Value_i]}\right] * Ranking$$

 $Overall Score_{j,i} = \frac{\sum_{m} Benefit Score_{j,i}}{\sum rankings}$ j = BMP Type i = Treatment Volume m = Evaluation factor

Can we summarize multiple evaluation factors in only one plot??

Example from Los Angeles modeling efforts





Results: Full Optimization

Full optimization

- 1 optimization curve
- Optimize all SCMs simultaneously
- Routing 25% of infilredevelopment to each SCM type



1000 Solutions across a range of AAFV and Costs



46

30 best solutions reach the AAFV target at the cheapest cost





Best solutions favor 1) bio-retention



Best solutions favor 1) bio-retention and 2) underground infiltration







Two best solutions:

- Cheapest Cost
- Smallest total capture volume required to reach baseline flow conditions
- One solution that favors bio-retention
- One solution that favors underground infiltration

Which is the optimal solution??





Stormwater Capture Outputs	1	2	
	Bio-	Underground	
Dominant SCM type	retention	Infiltration	
Cost	179,450	176,910	
AAFV *107 [ft ³]	2.1652	2.1665	

Very similar cost and average annual flow volume..








Life cycle costs and co-benefits may be the tipping point between SCM solutions.



Co-benefit Selection Process for i-DST



Future work and further analysis on two best solutions



"Greyer" Underground Infiltration Solution

	Score		
Stormwater Capture/LCC	33.3		
Life Cycle Analysis	0		
Co-benefits	0		
"Greener" bio-retention Solution Score			
"Greener" bio-retention Solution	n Score		
"Greener" bio-retention Solution Stormwater Capture/LCC	n Score 66.6		
"Greener" bio-retention Solution Stormwater Capture/LCC Life Cycle Analysis	n Score 66.6 100		



Thank you! Questions?



Email: emgallo@mines.edu

Email: idst@mines.edu

Website: https://idst.mines.edu/

Follow us: @iDST_Team

If you are interested in attending future workshops or beta testing the tool send us an email!!

idst@mines.edu

Matrix of all Optimization Controls



Integrating LID into Municipal Design Criteria – A Colorado MS4 Perspective



CITY OF BOULDER DESIGN AND CONSTRUCTION STANDARDS CHAPTER 7

STORMWATER DESIGN

Effective: June 20, 2019

CASFM

September 25th, 2019

Kevin Koryto, Stormwater Quality Engineer City of Boulder



Why talk about stormwater quality design criteria?







Water Quality Design Standards Included in MS4 Phase II Revised Permit:

1. Water Quality Capture Volume



Treat runoff volume for 80th percentile, 0.6-in event

2. Pollutant Removal



30 mg/L TSS effluent concentration

3. Runoff Reduction



Infiltrate 60% of the WQCV

Boulder's Design Criteria



The MS4 permit has new design standards...





Boulder Design and Construction Standards (DCS)

- Establishes technical requirements for infrastructure design & review
- Companion document to Boulder Revised Code (BRC)
- Not a manual, references external guidance documents
- First major revision in 20 years

CITY OF BOULDER

DESIGN AND CONSTRUCTION STANDARDS

CHAPTER 7

STORM WATER DESIGN

Effective: November 16, 2000

DESIGN AND CONSTRUCTION STANDARDS CHAPTER 7

CITY OF BOULDER

STORMWATER DESIGN

Effective: June 20, 2019

Boulder's Design Criteria



Implementation Timeline





The three elements of stormwater quality design criteria:

1. Implementation Threshold	<u>When</u> does stormwater quality have to be designed?
2. Design Standard	<u>What</u> is the metric for an adequate design?
3. Design Process	How are stormwater quality facilities designed?

Stormwater Quality Thresholds



Authority	Regulation	Specification		
Federal, EPA	Clean Water Act: NPDES Phase II Stormwater Final Rule (2003)	Implement permanent stormwater controls when more than <u>1 acre</u> of land is disturbed		
State, CDPHE MS4 Permits Replicate		Replicate		
Local	Local Ordinance	Replicate or go stricter		

Boulder's Previous Requirement: For less than 1 acre, city "may" require

Does the 1-acre threshold make sense for our community?

What about in built-out communities where redevelopment is predominant?

Stormwater Quality Thresholds



Innovative and Integrated Stormwater Management Date Published JUN 26, 2019 Resource Type SPECIAL REPORT

- Survey of 34 community stormwater programs
- 2/3 of communities have a threshold lower than 1 acre
- Increased impervious area and other conditions, also commonly used



55% Have a threshold < 10,890 sf **8%**

Have a threshold from 10,890 sf to < 21,780 sf

5%

Have a threshold from 21,780 sf < 1 acre

Have a threshold of 1 acre

27%

Have a threshold \geq 1 acre



Boulder's Approach

- Maintain 1-acre threshold
- Demonstration of LID required for all sites, including < 1 ac
- Collect development data to support future evaluation and revision

Low Impact Development (LID) Checklist						
LID techniques, as defined below shall be implemented for all new development and redevelopment						
LID techniques have been investigated and implemented to the maximum extent practical for this site.						
Conserve Existing Amenities: Planning efforts shall account for and, where practicable, preserve or restore existing site features that naturally retain stormwater on site, including vegetated areas, high infiltrating soils, and natural surface drainage patterns, such as meadows and trees.						
Vinimize Impacts: Planning efforts shall account for and minimize, where practicable, land disturbance, impervious surface addition, and soil compaction. This may include removing unnecessary impervious areas, minimizing driveway and sidewalk widths, and sequencing construction to minimize compacted areas						
Vinimize Directly Connected Impervious Areas (MDCIA): Planning efforts shall account for and minimize impervious areas, such as rooftops and pavement, that directly drain to the stormwater utility system or a local stream without prior stormwater control. This may include using or integrating						

pa receiving pervious areas into the site landscape, such as vegetated swales and buffers. Where practicable, site drainage patterns shall be designed to promote sheet flow to vegetated area and roof downspouts shall be disconnected from direct discharge to the storm sewer. Receiving pervious areas shall be designed to slow run-off and promote infiltration.

Design Standards



Authority	Regulation	Specification
Federal, EPA	NPDES Phase II Stormwater Final Rule (2003)	No specification
State, CDPHE	MS4 Permits	Establishes base design standards
Local	Local Ordinance	Replicate, select from, or go stricter

Design Standard Approaches:

- Pollutant Removal Requirement
 - Capture volume (WQCV*)
 - Effluent concentration limit
 - Percent removal
 - Load reduction
- Retention Requirement
 - Volume infiltrated or reused
 - Match pre-development hydrograph





MS4 Permit Design Standards:

- 1. Water Quality Capture Volume
- 2. Pollutant Removal





3. Runoff Reduction



Treat runoff volume for 80th percentile, 0.6-in event

30 mg/L TSS effluent concentration

Infiltrate 60% of the WQCV

For TSS Load Reduction: 60% infiltrated ≈ WQCV



Colorado's Phase II Permit Retention Requirement:

0.36 in = 60% of 0.6 in (roughly not accounting for WQCV calc)

National Survey (NYC, 2017):

1.0 in = Median retention requirement



Design Process



Authority	Regulation	Specification
Federal, EPA	NPDES Phase II Stormwater Final Rule (2003)	None
State, CDPHE	MS4 Permits	Review requirement
Local	Local Ordinance	Open ended

Local Criteria Building Blocks

Local Design Criteria

[Boulder DCS]

- Requires Standards & Methods
- Implements Process

MS4 Permit

[CO Phase II]

Design Standards

Recordkeeping/Review Requirements

Regional Manual [UDFCD Vol. 3]

- Technical Methods & Sizing Criteria
- Implementation Guidance

Boulder's Design Process Objectives



Objective	Driver			
	Colorado Phase II MS4 Permit			
 Meet Permit Requirements 	CDPS GENERAL PERMIT CORO09000 STORMWATER DISCHARGES ASSOCIATED WITH MUNICAL SEPARATE STORM SEWER SYSTEMS (MS4s)			
	EFFECTIVE JULY 1, 2016			
2. Promote Infiltration with GI/LID	Bacteria primary pollutant of concern <i>E. coli</i> TMDL Implementation Plan City of Boulder			
3. Guide effective designs	Lessons learned from design and maintenance failures			

Boulder Design Standard Hierarchy



Boulder Specified Design Approach		Infiltration Conditions			
Tier 1 – Runoff Reduction	Full Infiltration <i>Ex.: Bioretention without underdrain</i>	HSG A/B and No Risk			
Ļ					
	Partial Infiltration/Filtration <i>Ex.: Bioretention with underdrain</i>	HSG C/D and No Risk			
Ther 2 – WQCV	No Infiltration <i>Ex.: Bioretention with underdrain and liner</i>	Identified Infiltration Risk			
Ļ					
Tier 3 – Pollutant Removal	Alternative Design Ex.: Media Filter	Site Specific Constraint			



Selection Process

Treatment Approach	Step 1. Feasibility Screening	Step 2. Field Test Requirements		
Full Infiltration (Runoff Reduction)	HSG A or B, No soil, groundwater, or geological risk factors.	Required, Infiltration rate tested at ≥ 1 in/h OR For RPAs topsoil texture analysi		
Partial Infiltration (WQCV)	HSG C or D, No soil, groundwater, or geological risk factors.	Optional, Infiltration rate tested at < 1 in/hr OR For RPAs topsoil texture analysis		
No Infiltration (WQCV)	Lined system required due to risk factors.	N/A		
Alternative Design (Pollutant Removal/ Constrained Site)	Proof of physical site constraints/risk factor preventing other design.	N/A		

Boulder Infiltration Conditions





- HSG used only as a first look
- Differentiated soil conditions
 - HSG A: 16%
 - HSG B: 44%
 - HSG C: 28%
 - HSG D: 12%
- Boulder Creek Drainage: 80% HSG A/B



Process



Post-Construction Water Quality Design Form

4. Full Infiltration – Runoff Reduction Criteria

Ru eva dis No	Runoff Reduction Design Standard: SCMs are selected, designed, and constructed to infiltrate into the ground where site geology permits, evaporate, or evapotranspire a quantity of water equal to 60% of what the WQCV would be if all impervious area for the applicable development site discharged without infiltration. None of the applicable development area may be excluded when using the Runoff Reduction Standard.							
De	Design Criteria ^a					Yes	No	
1.	 Preliminary infiltration feasibility screening has been completed and documented in the drainage report with a rational conclusion for full infiltration. 							
2.	2. The Runoff Reduction Design Standard has been met for the treatment area.							
3. Required sizing criteria for full infiltration has been achieved and documented in the drainage report.								
4. Field infiltration test requirements have been met and documented in the drainage report.								
so	SCM NamebSCM TypecDrainage Area (ac)WQ EventWQ EventPercentSCM NamebSCM TypecDrainage Area (ac)Nolume (ft3)Volume (ft3)Volume (ft3)(%)			Percent Infiltrated (%)	Detention Storage (yes/no)			
Treatment Area Not Routed to SCM:					0	0%	N/A	
		Total:						

^aRefer to D.C.S. Section 7.16(D)(1) for full infiltration treatment approach criteria.

^bSCM Name shall match drainage report and plan designation. Attach additional table if more than three SCMs are planned.

^cSCM Type shall match USDCM Volume 3, Treatment BMP Factsheet nomenclature.



Building a green infrastructure (GI) program

City GI Program

GI Pilots, Guidance and Tools

Stormwater Design Criteria

Stormwater Permit Compliance

Acknowledgements



Heather Williams Mary Halley



Urban Drainage and Flood Control District

Holly Piza



wood.

Jeff Arthur Amanda Bevis Pieter Beyer Emily Halvorson Elizabeth Judd Kevin Koryto Scott Kuhna Brett Linenfelser Candice Owen Hella Pannewigh Jessica Pault-Altiase Edward Stafford

Thank You!



References:

Boulder Design and Construction Standards, Search: "Boulder DCS"

https://bouldercolorado.gov/plan-develop/design-construction-standards

Innovative and Integrated Stormwater Management, Search: "NYC Stormwater Report" <u>https://www.waterrf.org/resource/innovative-and-integrated-stormwater-management</u>

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Beneficial Use of Water Treatment Residuals as a Bioretention Media Amendment for Phosphorus Removal

2019 CASFM Annual Conference

September 24-27, 2019

- Basil Hamdan, P.E., CFM City of Fort Collins Utilities
- Tyler Dell Colorado State University –

Colorado Stormwater Center

















What are Water Treatment Residuals (WTRs)?

What are WTRs?

- The flocculating agent of water treatment
- Fort Collins Utilities uses Aluminum Sulfate Al₂(SO₄)₃
- Delivered as a liquid
- Stored in large tanks and added to the water





What are WTRs?

- Calculated, added, monitored for the source water
- Causes flocculation
- Removed in the sediment from water treatment residuals







What are WTRs?

- Sediment containing the aluminum sulfate as well as particles removed from water treatment needs to be managed (full-time position)
 - 1. Dewatering
 - 2. On-site Management
 - 3. Landfill Issues (Costs + Life Cycle)











Fort Collins Zero Waste Initiative

Meanwhile Back At The Ranch...

- Stormwater nutrients and current practices
- Regulation 85
- Lead pipes and phosphate
- COFC Climate Action Plan
- Innovate Fort Collins
- Colorado State University Stormwater Center - COFC Collaboration
- CSU- City Sustainability Network

Phosphorus Concentrations



WASTE

RESOURCE

Fort Collins Water Treatment Plant produces over 1,000 tons of WTRs/yr

Larimer County Landfill originally scheduled to be closed within 5-10 years

Regulation 85 sets new TMDLs for streams and phosphorus

Current stormwater practices export phosphorus


WTR Column Study

- Wanted to test the concept and mixing strategy
- 4 different configurations
 - Top Applied 0.5"
 - Top Applied 1.0"
 - Bottom Applied 1.0"
 - Mixed Application 1.0"



		Water Volume ft ³						
Year	# of Sig Events (> 0.1 in)	Total Precip	Sig Event	Total	Limited			
			Precip	Estimated	Estimated			
				Runoff	Runoff			
2007	21	104,280	83,490	66,165	17,753			
2008	22	110,220	98,670	80,520	24,405			
2009	41	173,333	155,760	121,935	41,310			
2010	28	115,253	101,805	78,705	26,175			
2011	32	143,633	127,958	101,558	33,735			
2012	18	72,765	59,483	44,633	17,940			
2013	33	143,055	127,793	100,568	34,680			
2014	33	131,753	107,828	80,273	32,970			
2015	38	155,595	134,558	103,208	36,435			
2016	27	93,473	75,983	53,708	23,348			
2017	36	139,343	117,398	88,110	33,383			
Average	30	125,700	108,248	83,580	29,285			

Hydrology Summary

Hydrology Summary

Based on previous rain gage analysis

- Fort Collins experiences an average of 30 runoff producing storms/year
- The average depth contained by the rain garden from those storms is 6.2"
- The 95th confidence interval 7.0"
- The average dissolved phosphorus EMC from the parking lot is approximately 0.228 mg/L





Performance of WTR Columns

- Bottom application performed the best followed closely by mixed
- The top applied did provide some removal but not much
- Application rates of top application did not impact performance
 - Time of exposure
 - Picking up pollutants from media beneath

Mix Type	Average Reduction				
Mixed 1"	71.9%				
Top Applied 1"	19.0%				
Top Applied 0.5"	19.3%				
Bottom Applied 1 "	77.1%				

WTR Pilot Project: 700 Wood Street



Site Background

Rain Garden (Bioretention)

- Installed in 2012
- 2.25 acre parking lot
- Rain Garden Area: 1,900 ft²
- Rain Garden Volume: 1,400 ft³







Stormwater Monitoring

Monitored site since 2013

Pilot Project Application

- Used a top application of approximately 0.5"
 - This should handle at least 10 years of phosphorus loading
- Installed sampling equipment to capture influent and effluent water quality samples
 - Looking primarily for dissolved P, aluminum, and potential radioactive particulate from the WTRs
- Collected a sample on 9/8/2019, results should be in soon...

Conclusions

- Mixed application appears to be the optimal application that would yield the least amount of risk
- Continuation of column study for 30 rainfall years
- Expansion to additional types of technologies
 - Wetlands
 - Extended Detention Basins
- Elimination/re-use of Fort Collins water treatment residuals waste

Thank you!

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@ bhamdan@fcgov.com



Educate, Enhance, Explore





CASFM 2019













Municipal Challenges

- Staffing resources
- Implementation funding
- Water quality/TMDL
- Fully built landscape





- Optimized BMP targeting
- Optimized BMPs by location
- Measurable, defensible outcomes



Benefits

- Comprehensive planning
- Apples-to-apples vetting
- CIP planning
- NPDES reporting support
- Powerful grant writing







Desktop Analysis: potential retrofit location screening

- Outfalls
- Pond mod's
- Open space

- Parking lots
- Rural
- Residential

- Urban
- Underground
- Base WQ Model



Field Analysis: site validation and data collection

- Confirm dataSite constraint
 - identification
- BMP alternatives identification
- Spot invert survey
- Infiltration testing
- Base model update



Treatment Analysis



- Subwatershed
 WQ modeling of alternatives
- 30-year, continuous modeling

- Incremental treatment (%) sizing runs
- TP, TKN, TSS, CU, PB, ZN, Hydrocarbons

- Water balance
- Treatment train effects
- WQ effects by outfall



Value Analysis



- Apples-to-apples comparison of alternatives
- 30-year Present Day Values (PDV)
- PDV / 30-year treatment = value

- Incremental cost analysis
- Triple Bottom Line analysis by subwatershed
- Alternative ranking by

subwatershed Comprehensive mapping and database development



Potential Locations

- Existing pond modification
 potential
- Above roadway culverts
- Below stormwater outfalls
- Within the conveyance system
- Transportation right of ways
- Large parking lots

- Hotspot operations
- Small parking lots
- Residential streets/blocks
- Open space/pervious areas for disconnecting pervious areas
- Large rooftops
- Underground treatment



BMP-Function Groups

- Extended Detention (Full Spectrum)
- Stormwater Wetlands
- Infiltration
- Bioretention
- Filtration
- Swales
- Stormwater Harvesting
- Chemical Reaction



BMP Groups by Location

	Stormwater Treatment Option (BMP)							
Location	Extended Detention	Wet Ponds	Stormwater Wetlands	Bioretention	Filtration	Infiltration	Swales	
Existing pond modification	•	•	•			0	0	
Above roadway culverts	•		•	0	0	0	0	
Below stormwater outfalls	•	•	•			0	0	
Within the conveyance system	A	0	•	•	0	0	•	
Transportation right of ways	•	•	•	•		0		
Large parking lots	•	•	•				0	
Hotspot operations	0	0	0	•	•	Х	0	
Small parking lots	0	0	0	•	•	•	•	
Residential streets/blocks	0	0	0	•			0	
Open space	0	0	0	•		•	•	
Urban hardscape	0	0		•				
Large rooftops	0	0	0	•			•	
Underground treatment		0	0		•		0	

Preferred stormwater treatment option

▲ = Feasible in some circumstances

 \circ = Seldom used for the retrofit

X = Not recommended under any circumstances

Source: Center for Watershed Protection











Runoff Volume by Land Use and Soils



Land Use/Soils



Total Solids Yield by Land Use and Soils



Land Use/Soils



Total Phosphorus Yield by Land Use and Soils





Metals Yield Index by Land use and Soils



Land Use/Soils



















₽¦ W	atersheds												\times	P
Help	SLAMM Calib	List	Add	Duplicate	Delete	Clear	Check	Cance	і ок					H
Se	lect Watershed					Waters	hed Nam	e S12	2-HDRN	A				
S12- S12- S12-	HDRNA INST -LDR	^		Outflov	v Device f	for Surfa	ice Runof	f 12				•		
S12b S12c	-LDR -LDR			Ou	tflow Dev	vice for F	ercolatio	n No	ne			•		
S120 S12e S12a	-LDR -LDR -LI					Total Ar	ea (acres)	21.4	076				
S120 S120 S12d	-LI -LI -LI			Pe	ervious Ar	rea Curv	e Numbe	r	6	9				
S12a S12b	-MDRNA -MDRNA			Indirectly C	Connected	l Imperv	. Fraction	י [0.0	41				
S12c S12d	-MDRNA -MDRNA			Scal	e Fractor	for Parti	cle Load	s [1					
S12a S12b S12c	-OSUD -OSUD -OSUD		Dir	ectly Connec	ted Impe	ervious A	rea Type	9	Vacuur	n Swept	I	Not Swept		
S12d S1e2	-OSUD -OSUD			Con	nected Im	nperviou	s Fractior	n [0			0.103]	
S12g S12g S12h	-OSUD -OSUD			D	epressior	n Storage	e (inches)) [0.0)2		0.01		
S12i- S12a	OSUD -PARK -PARK				Imper	vious Ru	inoff Coe	f	1			0.873		
S120 S120	-PARK -PARK			Sca	le Factor	for Parti	cle Load	s [1			1		
S12a S12b S12c	-SCOM -SCOM -SCOM			Imperviou	s Sweep	Frequen	cy (1/wk) [0					
S12a S12b	-SUB -SUB			Swee	ping Effic	iency Sc	ale Facto	r [1					
S120	-SUB -SUB								Sta	rt		Stop		
S120	-SUB	~			Vacuum	Sweepir	ig Seasor	; [10	1		1231		

P [®] Particle Parameters -									
Help Save File Read File Chec	k Cancel	ОК							
Particle File Name nurp50.p8p									
Particle Set Description NURP Particle Distribution - 50th Percentiles									
Size Fraction	1	2	3	4	5				
Description	P0%	P10%	P30%	P50%	P80%				
Accumulation Rate (lbs/ac-day)	0	1.75	1.75	1.75	3.5]			
Accum Decay Rate (1/day)	0	0.25	0.25	0.25	0.25]			
Washoff Coefficient	0	20	20	20	20]			
Washoff Exponent	0	2	2	2	2]			
Sweeper Efficiency (%)	0	0	0	5	15]			
Impervious Runoff Conc (ppm)	1	0	0	0	0]			
Pervious Runoff Conc (ppm)	1	100	100	100	200				
Pervious Runoff Exponent	0	1	1	1	1				
Settling Velocity (ft/hr)	0	0.03	0.3	1.5	15				
1st Order Decay Rate (1/day)	0	0	0	0	0				
2nd Order Decay Rate (1/d-ppm)	0	0	0	0	0				
Filtration Efficiency (%)	90	100	100	100	100				


Report:	Mass Balance	•	Term:	10 :	surface outflow	- Dec:	1 .
Device:	SS Treat-Infil	Var:	TSS	•	Transpose	Сору	Help

Term	Flow ac-ft	Load lbs	Conc ppm	Flow cfs	Load lbs/yr
01 watershed inflows	167.9	52198.9	114.4	0.3	62306.0
03 infiltrate	54.8	4209.8	28.3	0.1	5024.9
04 exfiltrate	54.8	0.0		0.1	0.0
05 filtered	0.0	4209.8		0.0	5024.9
06 normal outlet	91.6	15222.7	61.1	0.2	18170.2
07 spillway outlet	21.5	3677.3	62.8	0.0	4389.3
08 sedimen + decay	0.0	29087.4		0.0	34719.5
09 total inflow	167.9	52198.9	114.4	0.3	62306.0
10 surface outflow	113.2	18900.0	61.4	0.2	22559.5
11 groundw outflow	54.8	0.0		0.1	0.0
12 total outflow	167.9	18900.0	41.4	0.3	22559.5
13 total trapped	0.0	33297.2		0.0	39744.5
14 storage increase	0.0	0.0		0.0	0.1
15 mass balance check	0.0	1.6		0.0	2.0
Load Reduction %	0.0	63.8			
Mass Balance Error %	0.0	0.0			







Incremental Cost of Treatment





Within Subwatershed Value by Strategy

Alternative	Install Cost	Annual O&M	D.A. Load ¹ (TP-LB/YR) ²	TP Removed (%/YR) ²	TP Removed (LB/YR) ²	30-Year Treatment Value (\$/LB/YR) ²
Permeable Parking ³	\$140,400	\$200	10.6	78	9.8	\$14,347
Stormwater Planters ⁴	\$45,745	\$250	10.6	45	4.8	\$9 <i>,</i> 582
Sub-sidewalk Storage ⁵	\$195,000	\$250	10.6	42	4.4	\$44,375
Raingardens ²	\$15,000	\$250	8.3	34	2.8	\$5,446
Stormwater Planter Boxes ³	\$42,000	\$250	4.2	56	2.3	\$18,370
Infiltration Trench ⁴	\$2,500	\$0	2.1	79	1.3	\$1,923
H169 Detention ⁵	\$135,000	\$750	13.4	59	7.5	\$8,500
Extended Detention ²	\$225,000	\$750	67	46	31	\$2,299



Between Subwatershed Value by Strategy

Values Addressed

Subwatershed	BMP	Install Cost	TP lb/yr	30-yr TP value	Manage Stormwater	Surface WQ	Enhance Public Space	Infrastr. Integration
7	Infiltration Trench	\$2,500	1.3	\$1,923	Y	Y	Ν	Ν
6	Extended Detention	\$105,000	46	\$2,299	Y	Y	Y	Y
10	Raingardens	\$24,000	5.8	\$4,181	Y	Y	Y	Y
13	Raingardens	\$9,000	2.1	\$4,406	Y	Y	Y	Y
7	Raingardens	\$15,000	2.8	\$5,446	Y	Y	Y	Y
11	Stormwater Planters	\$14,000	1.8	\$7,917	Y	Y	Y	Y
7	H169 Detention	\$63,000	7.5	\$8,500	Y	Y	Y	Y
1	Stormwater Planters	\$45,745	4.8	\$9,582	Y	Y	Y	Y
12	Stormwater Planters	\$7,000	0.7	\$10,360	Y	Y	Y	Y
3	Stormwater Planters	\$49,000	4.6	\$10,707	Y	Y	Y	Y
1	Permeable Paving	\$140,400	9.8	\$14,347	Y	Y	N	Y
7	Stormwater Planters	\$42,000	2.3	\$18,370	Y	Y	Y	Y
1	Sub-Sidewalk Storage	\$195,000	4.4	\$44,375	Y	Y	N	Y



Contact Information



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H R G R E E N . C O M



Reimagining a Denver Landmark City Park Golf Course

CASFM September 25, 2019





Background







Background

PLATTE TO PARK HILL New Globeville Upgraded Landing **City Park** New 39th Ave **Golf Course Outfall Park** Greenway Cole Clayton Park Hill Downtown Denver - South Platte River City Park Neighborhood Neighborhood Golf Course Projected 100-Year Storm Flood





P2P Program







A Community Approach









Community Goals



























Constraints

- Groundwater
- Earthwork/Soil Suitability
- Hydrology
- Embankment
- Golf
- Trees
- Views
- Historic
- Schedule













Design Build Procurement

Why Design Build?

- Schedule
- Innovation
- One point of Contact











Procurement approach



Technical requirements



- 100-yr Storm detention
 - Q_{max}= 3,275 cfs (227 ac-ft)
 - 8hr max detention time
 - No adverse effects to golf in storm events
 - Integrated into golf course

Matrix MARTIN/MARTIN

- 10-yr storm impacts
- Mile High Flood District Maintenance Eligibility
- Water Quality residence time
- Trash Vaults offline



Final Design

- Design Overview
- Designing to the TR's
- Unique Design Features
- Construction Phasing
- 2D Model
- Lessons Learned







Design Build Team





Hale Drivin





PRARMATIC MEASTER

MEP

SSG

-















Design Overview









Meeting the Technical Requirements

	CPGC SWMM N	lodel	Return Period					
Comparison Table			WQ	10-Year	100-Year			
RFP	Qin	(cfs)	411	825	4398			
	Qout	(cfs)	294	419	3725			
	Volume	(ac-ft)	21	73	227			
	WSEL	(ft)	-	5241.1	5248.0			
Design	Qin	(cfs)	411	825	4398			
	Qout	(cfs)	276	419	3724			
	Volume	(ac-ft)	23.9	76.9	213.5			
	WSEL	(ft)	5239.1	5243.6	5251.0			

RFP Criteria Comparison	RFP	Design	
Non-Jurisdictional Dam Surface Area **	(acres)	20	14.2
Non-Jurisdictional Dam Volume Above Natural Grade	(acre-ft)	100	8.4
Non-Jurisdictional Dam Berm Height **	(ft)	10.0	0.6
Maximum Embankment Slope	(h:v)	4:1	4:1
Wetland Channel Length	(ft)	2000	2018
Max Wetland Channel Slope	(%)	0.3% - 0.7%	0.3%
Max Wetland Channel Velocity (Normal Depth) *	(ft/s)	3.5	3.5
Residence Time *	(min)	7-10	9.6
Maximum Forebay Depth	(ft)	4.0	3.0
Minimum Forebay Depth	(ft)	3.0	3.0
Forebay Sediment Depth	(ft)	1.5	1.5
Forebay Footprint	(acres)	1.0	1.2

*MAXIMUM WETLAND CHANNEL VELOCITY AND RESIDENCE TIME CALCULATED BASED ON NORMAL DEPTH CONDITIONS WITHIN THE TYPICAL WETLAND CHANNEL SECTION.

**MAXIMUM NON-JURISDICTIONAL DAM CRITERIA PER STATE OF COLORADO RULES AND REGULATIONS FOR DAM SAFETY AND DAM CONSTRUCTION. THE DAM CRITERIA IS MEASURED RELATIVE TO THE OVERFLOW WEIR CREST.



NOTES:

- CONTOURS SHOWN ARE ONE FOOT INTERVAL. EXISTING TOPOGRAPHY SHOWN WAS PROVIDED BY SAUNDERS CONSTRUCTION COMPANY.
- 2. HYDROLOGIC ANALYSIS IS BASED ON THE TWO BASINS DRAINAGE PLAN SWMM MODELING PROVDED BY MATRIX DESIGN GROUP ON SEPTEMBER 28, 2017. THE SWMM MODELING PROVIDED BY MATRIX IS THE BASIS FOR RFP ADDENDUM 3 AND WAS ADAPTED FROM THE MATT UDSWMM MODEL WITH CORRECTIONS BY ENGINUITY. PROPOSED POND ANALYSIS IS OUTLINED IN THE CITY PARK GOLF COURSE – REGIONAL DETENTION POND FINAL DRAINAGE REPORT, BY MARTIN/MARTIN, INC., DATED FEBRUARY 12, 2018.
- THE ADDENDUM 3, SECTION 12.2.4.11, AREA SPECIFIC DRAINAGE REQUIREMENTS THAT IDENTIFY PLAYABLE GOLF FACILITY CRITERIA, ARE SUMMARIZED AS FOLLOWS:
 - 3.1. NO PONDING GREATER THAN EIGHT HOURS OR ADVERSE IMPACTS SHALL OCCUR DURING THE 10-YEAR OR 100-YEAR RECURRENCE INTERVAL STORM EVENTS, ON ANY NON-GREEN PLAYABLE GOLF FACILITIES (FAIRWAYS, TEE BOXES, BUNKERS, CHIPPING AREA, PUTTING PRACTICE AREA, CART PATHS, GOLF CART BRIDGES, FIRST TEE FACILITIES, ETC.). ADVERSE IMPACTS ARE DEFINED AS EROSION, DAMAGE TO TURF, WASHING AWAY OF MATERIAL, OR ANY OTHER DAMAGE THAT WOULD REQUIRE MORE THAN MINIMAL MAINTENANCE. CONSIDERATIONS TO DETERMINE ADVERSE IMPACTS SHOULD INCLUDE DEPTH OF WATER, VELOCITY OF FLOW, AND TYPE OF FACILITY BEING INUNDATED.
 - GREENS SHALL NOT BE INUNDATED DURING THE 10-YEAR NOR THE 100-YEAR EVENT.
- GROUNDWATER SURFACE ELEVATIONS ARE BASED ON DATA FROM THE INTERIM DRAFT GEOTECHNICAL INVESTIGATION REPORT. CITY PARK INVESTIGATION, BY YEH AND ASSOCIATES, INC. JANUARY 11, 2017.





















Trash Vault Diversion









Construction Phasing





























Lessons learned

- Trash Grate diversion
 - Maximize grate area
- Neighboring projects
- Balancing stakeholder goals
 - Communication
- Phasing



